

The Fire Resistance Performance of Concrete Columns With Different Compressive Strength

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Research Article

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Posted Date: December 21st, 2021

DOI: <https://doi.org/10.21203/rs.3.rs-1051498/v2>

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The Fire Resistance Performance of Concrete Columns with Different Compressive Strength

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ABSTRACT

Four full concrete columns have been created Tested below high temperature for The fire resistance of concrete elements in concrete with particular compressive strengths. The standard concrete with compressive strength values of C25 were made of one of the four specimens, while the rest were made of C35, C60 and C75 respectively, respectively. During simulation of Within the laboratory furnace, the same For the specimens, axial forces were applied. Many experimental outcomes parameters were evaluated in contrast, including temperature changes, Vertical moving, side deflection, fire resistance and Failed properties of the specimen. The results have shown a rise in the compressive strength of the concrete for the concrete columns from the outside up to the inside of the column the same cross section of the lower compressive forces of concrete display better fire resistance efficiency with the same initial axial strength ratio. The C35, C60 and C75 columns' fire resistance is higher than standard concrete columns. The initial and secant rigidity of the columns of Reinforced concrete (RC) has also The percentage decreased dramatically after fire exposure and the temperature increased from 25 to 750 ° C.

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1. Introduction

Many researchers have been studying the behavior of Reinforced Concrete (RC) exposed to fire over the last two decades. Understanding the column behavior at high temperatures helps to assess the fire protection degree of the structure. The overall load for a column exposed to fire was indicated to be 20-40 % Lower than the column before the fire [1]. A major decrease in the elasticity module was also observed at high temperatures which showed a 62-72 percent decreases after heating up to 400 ° C. Several experiments have been carried out based Concrete part fire output. Lie et al. [2,3] and other Canadian citizens in the 1990s researchers conducted studies with fire resistance, primarily on standard Filled RC columns and tubular steel columns with changed pieces of concrete, available their formula for fire resistance, now used by the global construction industry. In Kodur et al. [4,5] and Iguchi et al. [6] regular concrete RC beams and columns were studied fire resistance. Gernay et al. [7] structural performance of natural fires in concrete columns was investigated.

The modern form of environmental-friendly material recycled aggregate concrete (RAC), using waste concrete to substitute natural aggregates for recycled materials, is rendered partly or completely by washing, grinding, grading and mixing methods. Sarhat et al. [8] examined RAC's residual mechanical response following high-temperature exposure. However, only when the RC column fire resistance efficiency with different intensities was investigated, few fire resistance experimental concert studies of RC structures and materials perforated. The principal structural elements of most buildings are vertical columns and the loss of load-bearing capacity in the fire causes the entire structure to be partially or entirely demolished. Work on the efficiency of RC columns to protect life and property in fire situations is thus important. The results show that specimens with a high column have a lower temperature than standard concrete, load power, strength and fire conditions in the same place. Fire endurance decreases as specimen strength increases.

In this paper, On the basis of experimental data, the efficiency of fire resistance was evaluated of three RC columns and one Standard RC columns of specific compressive concrete forces. in order to give an experimental basis for the use of the design of fire-resistance using different concrete compressive strengths.

2. Resources and goals

The debate above indicates that ductility and energy absorption and column rigidity are important, because safety is guaranteed in these columns when natural disasters happen. The ductile behavior of the columns before and after the fire is more important during the fire as at the beginning of the column's vital sections it provides a visible warning, with no severe resistance loss, which in effect provides the building a time to safe evacuation and to ensure the protection of the fire-fighters. Until now, however, only relatively few studies have concentrated on Ductility study, energy absorption potential and column rigidity. Data showing the status of these characteristics after the fire has been released are also lacking. This study aims to collect and raise knowledge in this area in order to address the gaps in science. The main objective of such work conducts investigational analysis to investigate Effect on ductility, energy efficiency and steepness characteristics of columns of RC exposed to fire and exposed to stress on twoopposite sides.

3. Materials and manufacturing

The optimum RC mixing proportion is shown in table (1), which was selected for the production of RC with specific compressive strength following multiple test mixes and ensuring workability for compliance with the ASTM C1437-15 requirement [9]. For this study [20], products that cast off included cement [18], sand, gravel [19], steel enhancement and traditional water processing with concrete admixtures (superplasticizer's) were used [20].

3.1 Experimental Program of Structural Behavior

Structural behavior study method consists of casting and testing 4 specimens. The columns of RC were Cast of different abilities. Each of 4 specimens (NC) with concrete mixtures has compressive cylinder strength of approximately 25, 35, 60 and 75 MPa respectively. Tables (1) , (2) and (3) demonstrate the mixing proportions, fresh properties and strength of these blends, and characteristics of the steel reinforcing bars. In other words, the key parameters in this analysis are concrete form and compressive power.

Table (1): Mix proportion of concrete.

Concrete type	Mix symbol	Cement (kg/m ³)	Water (kg/m ³)	Sand (kg/m ³)	Gravel (kg/m ³)	SP/C %by wt.
NC	NC25	350	181	700	1155	-
	NC35	420	181	700	1155	-
	NC60	465	175	675	1115	0.35
	NC75	570	159	660	1085	0.70

Table(2)Results of Mechanical Properties of Hardened Tests

Column symbol	f'c MPa[21]	fr MPa[22]	ft MPa[23]	Ec GPa[24]
RC25	25	3.68	2.65	15.54
RC35	35	5.145	3.71	20.79
RC60	60	8.91	6.43	35.54
RC75	75	11.08	7.87	43.79

Table (3) Properties of bars of steel *

Diameter of the nominal bar (mm)	Zone of the Bar (mm²)	Modulus of Elasticity (GPa)	Yield Stress (MPa)	Strain at Yield Stress (mm/mm)	Ultimate Stress (MPa)	Strain at Ultimate Stress (MPa)	Elongation (%)
8	50.27	202	545	0.0027	660	0.18	15.5
12	113.1	201	503	0.00252	615	0.13	14.5

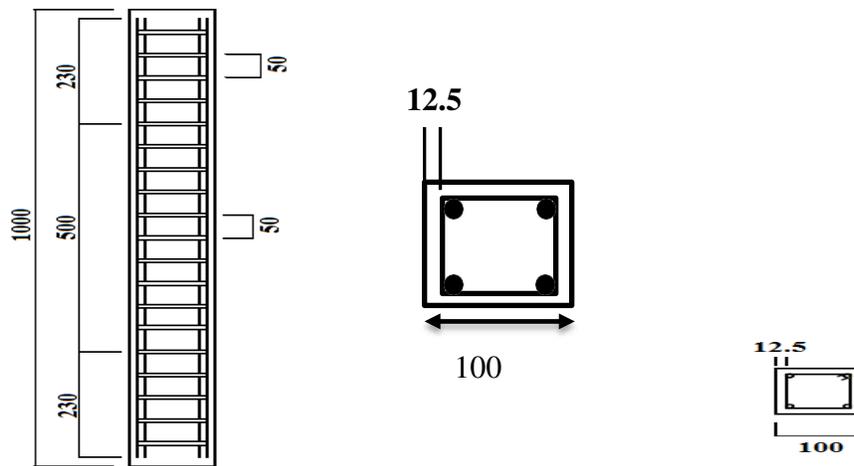
*Ø8mm manufactured by Turkish company

*Ø12 manufactured by Ukraine company.

*Tests were carried out by testing laboratory in the College of Engineering, University of Kufa

3.2 Column Mould and Strengthening Column Steel Bars

The mold was arranged from plywood with internal dimensions (100 to 100 to 11000) mm in order to cast column samples and it was designed according to the ACI 318M-11[25] design code. Four longitudinal steel reinforcement bars were used at the cross section of each column specimen corner. The longitudinal steel support bar diameters have been used (12 mm). Even stirrups were deformed steel bars with (8 mm) diameter. The column strengthening details are shown in Figure (1).



(b) Column with ties spacing 50mm

All dimensions are in mm

Fig. (1): Details of columns reinforcement.

3.3 Testing procedures

The fire experiments were performed by casting off a furnace made Refractive brick and refractive mother in size (15000 = 1500/1250 μm) of 250 mm wall thickness. The combination comprises four liquefied petrol gas burners on each side of the fire to spread the fire equally over the column length. (2). The gas control unit and the heating pads were connected to a wireless gas gage in order to monitor the gas discharge, to maintain a constant firing temperature at a target temperature set in the digital gage prior to starting the fire test. The furnaces are a metal steel structure made of three main components, a loading frame, a firing cage with insulation caps like figure. Furnace is constructed from metals and is a steel structure. (2), by means of a lever crane, the column manually positioned in the furnace. Fire exposed columns. After exposure to flames, columns were tested with a 500-ton hydraulic loading machine and accuracy of 0,5 tons in the Concrete testing laboratory, At a temperature rate of 2 ° C / s the temperature measured slowly increased until the target temperature was surpassed by fire-temperature. The test configuration and instrumentation details are shown in Fig. (2) and (3).

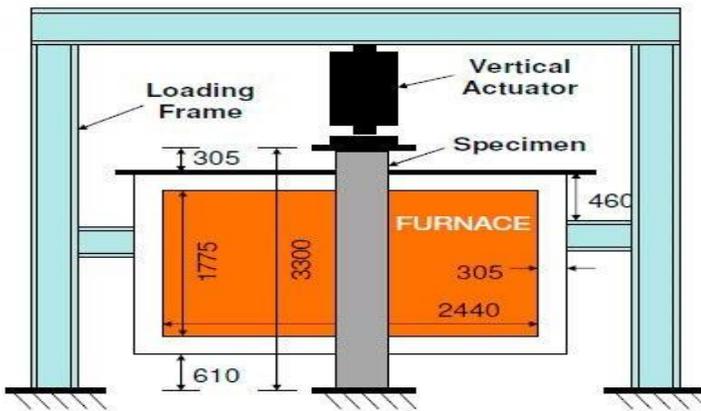


Fig. (2):Fire test setup (dimensions in mm)

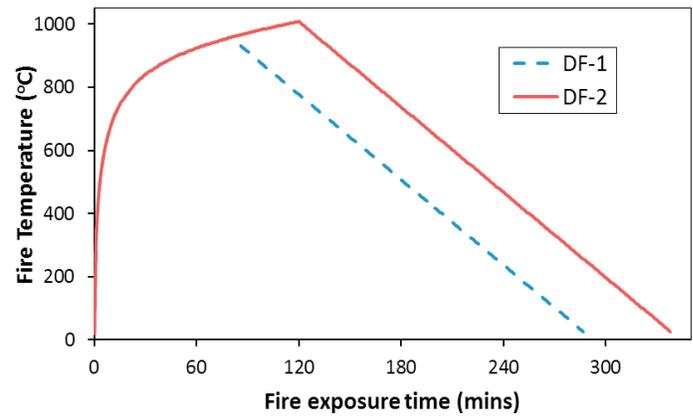


Fig. (3):Design fire scenarios.

4. RESULTS AND DISCUSSION

4.1 Load-deflection results

The displacement phenomenon under load is very important to calculate as the temperature rises to understand and assess the behavior of the fire exposed column. As can be seen in the figures, the displacement increases with an increase in temperature. The deflectometer dial measurements were used to monitor Axial and sideways specimen deformation. As shown in Figure, the demec points were placed For axial and lateral deformation pressure readings and dial gages. (3). The load was applied at a load rate of 1 kN / s using a load sensor. For this work two dial gauges with a precision of 0.001 mm per deviation were used. Such dial gages The lateral deformation was placed on a column-type electro-hydraulic tester piston at Halfway along the central vertical lines of the column specimens as shown in figure. (4) to (7), respectively.

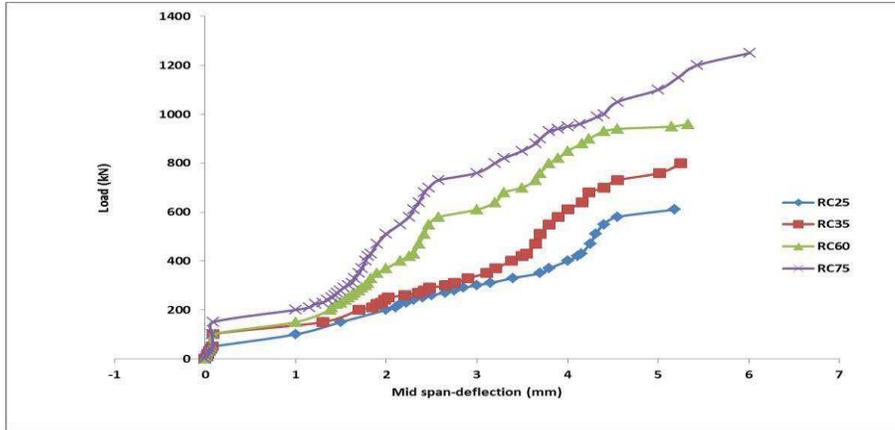


Fig. (4): Relation load-deflection for columns temperature 25°C.

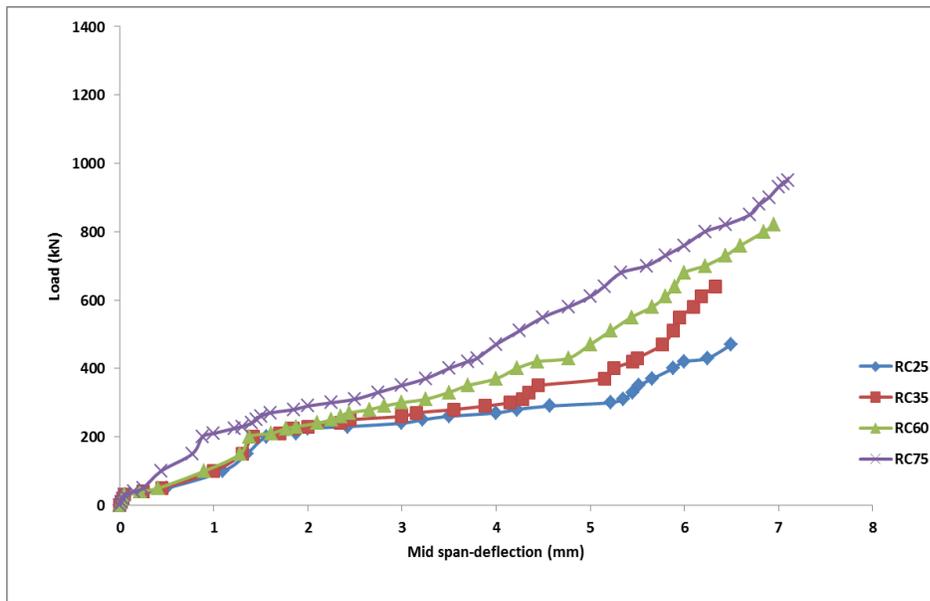


Fig. (5): Relation load-deflection for columns temperature 400°C.

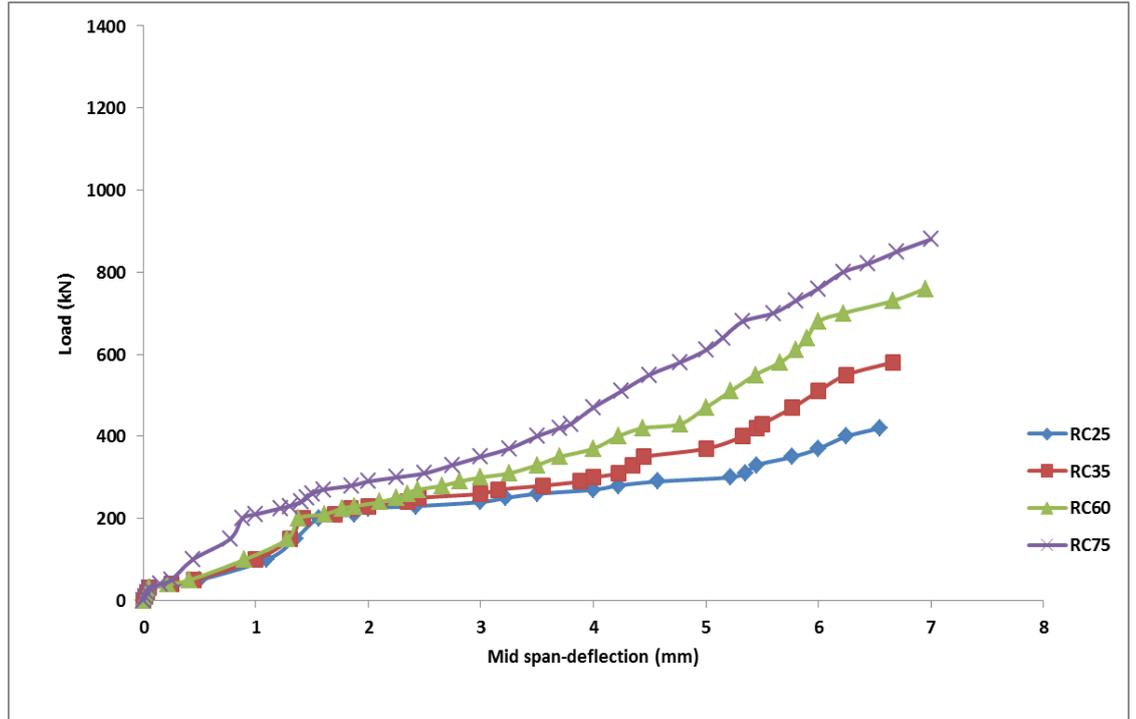


Fig. (6): Relation load-deflection for columns temperature 600°C.

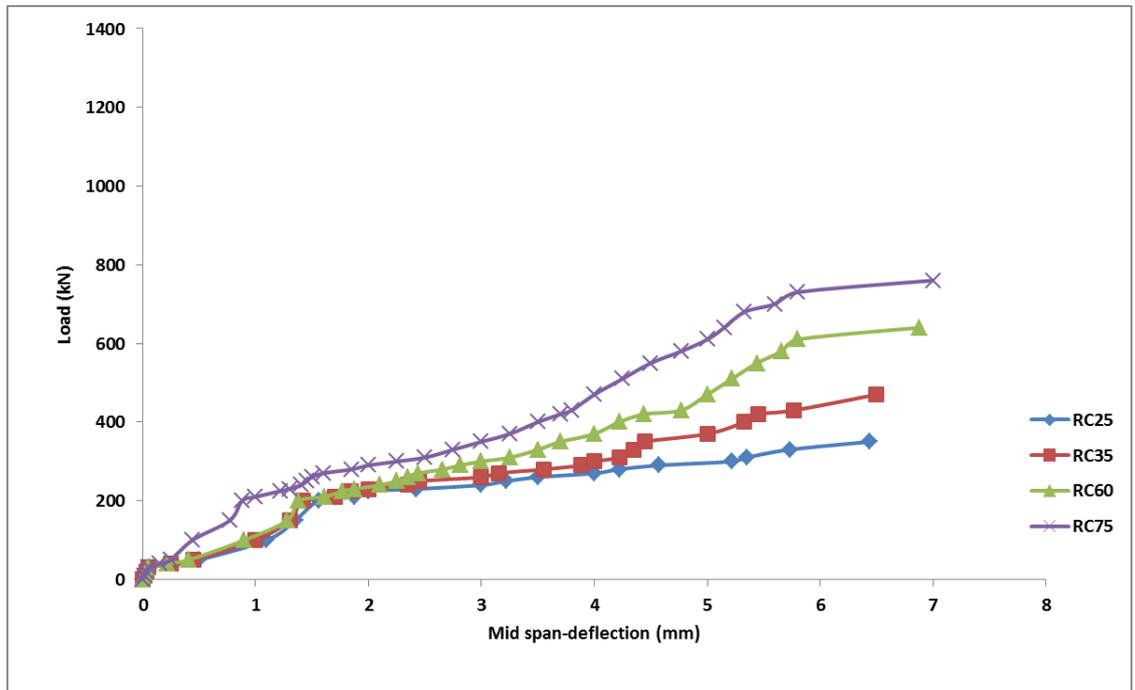


Fig. (7): Relation load-deflection for columns temperature 750°C.

5. Conclusions based on the results of the experiment:

The findings can be taken from the results of the parametric research discussed in the present paper using both experimental analysis for columns subjected to axial loads.

1. The index of the ductility displacement is not useful in determining the impact of fire exposure on RC column ductility in which displacement ductility index values have not been significantly affected following fire exposure.
2. In assessing the situation of ductility of RC Fire exposure columns, measurement Capacity of energy absorption has been demonstrated.
Results of the test indicate that RC columns missing approximately (33 – 47%) Potential for energy absorption after exposure to the fire at about 400 ° C and (55-63%) after exposure to the fire at about 750 ° C.
3. Results of the test indicate that lateral addition connections and the increase of Layer of concrete is not necessary to increase the RC columns' ability to absorb residual energy after fire exposure to 750 ° C.
Although The energy absorption potential of RC columns with a wider concrete cover is significantly decreased at 400 ° C.
4. Fire temperatures increased from 400 to 750 ° C decreased concrete pressure strength and improved Ductility, absorption of energy and RC column rigidity features.
5. After concrete cracking, the deformation of high-performance cement columns increases as the strength of the concrete is increased.
6. As the fire spreads, cracks spread and the cement is broken, followed by a large explosion with some local longitudinal reinforcement, particularly in high cemented columns.
7. The paper suggests a new method to calculate the degraded columns ductility index following fire exposure based on their energy consumption efficiency.
8. The results from the exploration analysis indicate that Containing and original rigidity decreased dramatically after fire exposure, and that the fire temperature rose by a higher percentage when the temperature increased between 400 and 750 ° C.
9. Additional work needs to be performed Research the after fire activity of RC columns with standard fire curves after fire exposure at temperature levels above 750 ° C because, due to limited column dimensions, the results The fire temperatures of this work are limited to up to 750 ° C.
10. The pinnacle strength of the concrete columns significantly increases with the compressive strength of the concrete improving the ductility, energy absorption and rigidity of the RC columns.
11. Reduce the crack size as the compressive strength of the concrete increases.
12. The overall stress in RC columns depends on the compressive strength of the concrete and the elevated temperature of the fire from 400 to 750 ° C.
13. The findings of the final factor analysis were very consistent with the results of the experiments, the higher columns of lateral strain supported by larger bar diameters and vertical displacement were similar to those of Smaller bar diameter reinforced columns.

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